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7. Abstract

This description of work directs field activities for performing several aquifer tests at and near the 200 West Area in support of the 200-UP-1 Project. Tests will consist of slug tests, and single-well and possibly multiple-well constant discharge tests. Test results will be used to expand and refine the hydrogeologic conceptual model of the 200 West Area.

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1.0 INTRODUCTION

This description of work (DOW) details the field activities associated with aquifer testing in the uppermost unconfined aquifer beneath portions of the 200 West Area. The focus of aquifer testing is primarily the U-1 and U-2 Crib areas near the uranium, technetium, and nitrate Interim Remedial Measure plumes beneath the U Plant Aggregate Area, although testing will also cover a broader portion of the 200 West Area to supply data needs. This DOW serves as a test plan for those performing the work. It should be used in conjunction with the Remedial Investigation/ Feasibility Study Work Plan for the 200-UP-1 Groundwater Operable Unit, Hanford Site, Richland, Washington (DOE-RL 1992a) for general investigation strategy and with Environmental Investigations and Site Characterization Manual (WHC 1988c) for specific procedures.

The overall work scope for aquifer testing is defined in Sections 4.2.3 and 5.4.2 of the 200-UP-1 Operable Unit Work Plan. Since existing wells were not found to be suitable for determining vertical communication between aquifers, it was decided to carry out aquifer tests to provide hydraulic conductivities for the uppermost aquifer only (i.e., the Ringold gravel unit E). This effort will fill in hydraulic conductivity data gaps and thereby support refinement of the conceptual model. A more rigorous testing program will be initiated after installation of the planned cluster wells to determine the degree of aquifer intercommunication as described in the work plan.

1.1 SCOPE

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This DOW provides technical and administrative guidance for performing slug tests and possibly constant discharge tests in the uppermost aquifer system beneath the 200 West Area. Specific items included in the DOW are test design requirements, field operational requirements, implementation requirements, and data collection guidelines for the aquifer testing. The DOW was prepared in accordance with Environmental Investigation Instruction (EII) 1.14, "Preparation of Descriptions of Work" (WHC 1988c).

The field testing will consist of at least a slug test, a step-drawdown test (as needed), and in some cases a constant discharge test. The slug tests will be used to provide estimates of hydraulic conductivity in areas where little or no data are available, to confirm previous slug test results, and as a check against hydraulic conductivities determined from the constant discharge tests. Step-drawdown testing will be used to determine an optimum discharge rate for the constant discharge test. Constant discharge tests will be performed to estimate the hydraulic conductivity of the aquifer. These tests will usually be single-well tests, but when possible observation wells will be used.

Aquifer tests are planned at seven wells (Table 1 and Figure 1). The reasons for selecting these particular wells and locations are described in Sections 1.2 and 1.3. All the aquifer tests will be conducted in the Ringold Formation gravel unit E. A more extensive aquifer testing program will be implemented later, after the installation of the proposed cluster wells. Field activities are expected to commence in July 1993 and be completed by August 1993. A site visit will be made to each test site prior to testing to determine the suitability of each well for aquifer testing. If a well is not

suitable, the tests may be omitted. Another well may be substituted in this case, subject to all the requirements specified in this document.

1.2 DATA QUALITY OBJECTIVES

The primary data quality objective for the aquifer tests is to provide estimates of hydraulic conductivity to be used in the groundwater numerical model (DOE-RL 1992b). The hydraulic conductivities will enlarge the groundwater model database by filling in data gaps (i.e., areas where hydraulic conductivity values are currently unavailable), or confirming hydraulic conductivity values determined from previous tests. Specific data quality objectives for aquifer testing are listed in Table 4-4 in DOE-RL (1992a); specifically, hydraulic parameter estimates are constrained by the limitations of available testing methodologies to approximate order-of-magnitude values.

The field program generally will consist of single-well constant discharge tests to evaluate aquifer hydraulic conductivities. This test method is expected to yield values of transmissivity that approach the true aquifer transmissivity. At sites where observation wells will be used, it may be possible to estimate other aquifer parameters, such as the vertical hydraulic conductivity and the storage coefficient. These later parameters will also be used in the groundwater model.

1.3 SITE SELECTION CRITERIA

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In general, the test wells and locations were based on the data requirements of the groundwater numerical model (i.e., to confirm hydraulic conductivities and to fill in data gaps). Prominent consideration was also given to the area around the U-1 and U-2 cribs where the interim remedial measure is planned. However, the lack of existing wells in the U crib area precluded any testing directly downgradient.

Existing wells were used in all cases because of resource and time limitations. The installation of new characterization wells is costly and requires significant lead time to plan for drilling and aquifer testing. The selection criteria can be summarized as follows.

- 1. Only existing wells will be tested.
- 2. The wells must be screened or perforated in the top of the unconfined aquifer so that the data collected at each site will be consistent with other aquifer test results in the model database.
- 3. The test results should provide confirmatory values of hydraulic conductivity for data in the groundwater model database (which consists of primarily slug test results).
- 4. The test results should fill in data gaps for the groundwater model.
- 5. Testing will be performed primarily in the eastern portion of 200 West Area, which is the general direction of contaminant movement from the 200 West Area and the primary area of concern for the groundwater model.

- 6. Better definition of the lateral extent of an east-west trending high hydraulic conductivity zone just north of the U-1 and U-2 cribs is desired.
- 7. Multiple wells should be used if possible, which generally provides more representative estimates of hydraulic conductivity, and may also supply additional parameters (vertical hydraulic conductivity and the storage coefficient) that can be incorporated into the groundwater model.

Using these criteria, seven test locations were selected (Table 2). Additional sites (to-be-determined wells) could be selected later if time permits, sufficient resources are still available, and other important data needs for the groundwater model are identified. Appendix A contains information on the construction of these wells.

A well assessment will be performed at each proposed test site prior to testing to determine the suitability of each well for testing. If one or more of the proposed wells is inadequate for testing, alternative locations may be chosen. Alternate wells will be chosen according to the criteria listed above and tested using the same general approach described in this DOW.

Table 1 lists the current proposed wells and general well completion information for each of the aquifer test wells. Figure 1 shows the locations of the wells in the 200 West Area. The nonpumping wells listed in Table 1 are observation wells, which may be close enough to a pumping well to warrant monitoring during the testing activities.

1.4 TESTING LIMITATIONS

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Some of the aquifer tests will consist of single-well constant discharge tests. In one case (or maybe two), a multiple-well test will be performed. The multiple-well test(s) may furnish information on the vertical hydraulic conductivity and the storage coefficient of the aquifer. Some general test limitations are identified below.

- The test results will only apply to the top of the unconfined aquifer and should not be considered representative of the entire saturated thickness. This restriction is chosen by design to ensure consistency with past test results. Most historical aquifer testing was performed in the top of the unconfined aquifer, and it is data from these tests that support the groundwater model. This restriction is also a necessity because most of the wells in the 200 West Area are completed in the top of the unconfined aquifer, and aquifer testing will be conducted using existing wells.
- Aquifer testing will not be site-specific to the U-1 and U-2 cribs, which is the source of the several contaminant plumes and the attention of the interim remedial measure. Even though this was a criterion considered for selecting test wells, very few wells are present in this general area. Resource limitations precluded the installation of characterization wells near the cribs.

The estimated hydraulic conductivity for the single-well tests will be approximations of the true hydraulic conductivity, because several key assumptions of the single-well test analysis are violated under the set test conditions. One significant assumption is the requirement for a fully penetrating well screen (or perforations). Analytical methods are available to handle this variation, but only for multiple-well tests.

Even with these limitations, the aquifer test results are expected to provide reasonable estimates of hydraulic conductivity for the top of the unconfined aquifer. Given (1) the proposed intent of the groundwater modelling effort (an evaluative/scoping tool); (2) the relatively large area covered by the model; and (3) the use of a plan-view two-dimensional model, the hydraulic conductivity data should meet the level of detail required by the groundwater model.

In addition, it is currently perceived that most of the contamination from the U-1 and U-2 cribs is situated at the top of the unconfined aguifer. If this is a correct assumption, then modeling the top of the unconfined aquifer is a reasonable effort for remedial alternatives.

2.0 GENERAL REQUIREMENTS

2.1 REQUIRED DOCUMENTS AND PROCEDURES

All work will be performed in accordance with the following documents and procedures:

- WHC-EP-0383, Environmental Engineering, Technology, and Permitting Function Quality Assurance Program Plan (WHC 1990a)
- WHC-CM-7-7, Environmental Investigations and Site Characterization Manual (WHC 1988c)

 - EII 10.1, "Aquifer Testing"
 EII 10.2, "Measurement of Groundwater Levels"
 EII 10.3, "Purgewater Management"
 EII 6.1, "Activity Reports of Field Operations"
 EII 6.2, "Resource Protection Well Services."

2.2 HEALTH AND SAFETY

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All personnel working to this description of work will have completed the 40-hour Hazardous Waste Site Worker training program and will perform all work in accordance with the following:

- WHC-CM-4-10, Radiation Protection (WHC 1988e)
- WHC-IP-0692, Health Physics Procedures Manual (WHC 1991)
- WHC-CM-4-11, ALARA Program (WHC 1988a)

- WHC-CM-4-3, Industrial Safety Manual (WHC 1987)
- WHC-CM-7-5, Environmental Compliance Manual (WHC 1988b)
- Site-specific health and safety plan or job safety analysis.

3.0 TEST REQUIREMENTS

The following subsections describe specific requirements for the aquifer tests such as determination of pumping rates; the selection of the pump; the equipment setup; the length of each test; and baseline, pre-test, and post-test monitoring.

3.1 TEST TYPES

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Ideally, hydraulic conductivity estimates from the slug test data should equal values calculated from the constant discharge test. However, slug test results are readily affected by near-borehole conditions that may not be representative of overall aquifer properties. Influences such as drilling effects, well completion activities, and natural formational heterogeneities can contribute to variations in the hydraulic conductivity. Slug tests are readily influenced by these factors. For this reason, constant discharge tests are planned along with the slug tests to confirm and estimate the hydraulic conductivity of the aquifer.

The ideal testing sequence will consist of an initial slug test (at the pumping well and all observation wells), a step-drawdown test, a constant discharge/recovery test, and a final slug test. The slug test and step-drawdown tests will provide preliminary information for planning and conducting the constant discharge test, including an initial estimate of hydraulic conductivity and an optimum rate of discharge.

At most of the wells, neither the hydraulic conductivity nor the transmissivity has been determined. One exception is well 299-W22-41, where the transmissivity was estimated to be 140 ${\rm ft}^2/{\rm d}$ from a previous slug test (WHC 1992). A constant discharge test has not been performed at this well.

At well 299-W22-41, and possibly 699-37-82A, observation wells will be available, making these multiple-well tests. Some historical information is available for aquifer testing in well 699-37-82A, reported in Graham et al. (1981). An evaluation of the Graham test data will be performed to determine if an additional aquifer test is necessary at this site.

The primary objective for the aquifer tests is to determine aquifer hydraulic parameters. For single-well constant-discharge tests, only the hydraulic conductivity (transmissivity) can be determined (an inherent limitation of the method). For multiple-well test(s), additional parameters such as the vertical hydraulic conductivity, the specific yield, and elastic storage coefficient may be estimated.

Slug tests will provide an initial estimate of the aquifer hydraulic conductivity. The slug test conductivities will be compared to conductivities calculated from the constant discharge test to help evaluate the correspondence between these test types.

A pre-test site visit and well assessment is planned to verify the condition and adequacy of the older wells for testing and to check the status of the newer wells. During the well assessment the following tasks should be performed:

- Measure the depth to water
- Tag the bottom of the well
- Note any obstructions in the wells
- Draw a well-head diagram (a Polaroid picture is also recommended)
- Measure the distance to nearby wells (which should be located within 100 ft of the pumping well).

If the initial evaluation of the wells indicates that testing is not feasible (i.e., there is sand fill-up, the desired interval is not accessible, or hydraulic head elevations are not distinct in wells completed as piezometers, etc.) the well will be eliminated as a test site, and no further evaluation is required. Alternate test wells may then be chosen to replace disqualified wells. Potential alternate well sites are indicated on Table 1. If the evaluation does show the well to be viable, the well may be further assessed by running a camera survey.

3.2 SEQUENCE OF FIELD ACTIVITIES

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Field activities at each site will begin with the well assessment and, if favorable, end with the final slug test. Figure 4 is a generalized flow chart placing each activity in chronological order. Some of the field activities can be completed as a group: for example, most of the well assessments can be finished before any aquifer testing begins.

Aquifer testing will be initiated only after several administrative tasks are completed. These tasks include the following:

- An environmental assessment to determine the impact of purgewater on endangered, threatened, or sensitive plant and animal species if water is to be disposed of to the ground (only outside the 200 West Area fence)
- A job safety analysis to determine the safety requirements for each site.

3.3 DISCHARGE RATES AND PUMP SELECTION

The primary purpose of the step-drawdown testing will be to estimate the optimal discharge rate for the longer term aquifer test. Two to five steps at

60 to 90 min each may be necessary to make this determination. A reasonable drawdown for the long-term test would be more than 5 ft in the pumping well, not exceeding 50% of the screen or perforated length, and at least 2 to 3 ft in the observation well (if available and nearby).

The final selection of the pump for each well will depend on the results of the step-drawdown tests. An initial best estimate of the discharge rate (and therefore pump selection) will be based on the estimated hydraulic conductivity from the slug test. The pump used for the step-test will be selected from drawdown projections based on the slug test results.

The pump (or riser pipe) should have a backflow (check) valve to prevent water in the pipe draining back into the aquifer after the pump is shut off.

3.4 EQUIPMENT SETUP

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The wells will not require any structural modifications for aquifer testing. It may be advantageous to install a packer in the observation well during slug testing in the pumping well to eliminate wellbore storage effects, and thereby increase the measurement sensitivity.

For the step-drawdown tests and constant discharge tests, the pump should be installed within 5 ft of the bottom of the screen, or at a depth that is at least 3 to 10 ft below the level of maximum expected drawdown. This setting should provide an adequate buffer to prevent cavitation.

A calibrated transducer will be used in the pumping and observation wells for baseline monitoring, pre-test water-level monitoring, and during recovery monitoring. The transducer depth setting, the recording frequency, and the calibration requirements are contained in EII 10.1, "Aquifer Testing."

A laboratory or field calibrated flow measurement device (which may be an orifice) will be used to monitor the discharge rate. The measured error of the flow measurement device must not exceed $\pm 10\%$ of the total flow.

If a rotor meter-type flow meter is used for low flow rates (<10 gal/min), the factory calibration is acceptable. The flow rate must be confirmed with a stop watch and container of known volume while running the test.

Field checks must be made to confirm proper operation of any flow measurement device. One useful field check would be using the weighing tank method, where the weight change of water is measured over a specific period of time.

Flow rates should be recorded about every 5 min at the start of the test, and at a maximum of 30- to 60-min intervals after the first 30 min. If a transducer can be used for recording flow rates (as with an orifice), the rate should be set to a logarithmic recording frequency at the start of the test with a maximum rate of every 30 to 60 min.

Flow measurement devices must be installed with the correct length of straight run pipe upstream and downstream from the device per the manufacturers recommendation. Expected flow rates in the 200 West Area are

expected to range from 1 to 50 gal/min based on the estimated hydraulic conductivities (Figure 2).

3.5 TEST LENGTH

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In general the constant discharge tests should run until the effects of delayed gravity drainage are minimal and a straight line is defined on a semilog plot of time versus drawdown. It is anticipated that the test will run from 4 to 8 h, but may require as long as 1 day, depending on aquifer conditions. Final determination on the length of the test is at the discretion of the Aquifer Test Lead. The rationale for stopping the test will be recorded on the field activity report.

At wells where the transmissivities are relatively low and a larger diameter casing is present (8 in. or greater), borehole storage effects will dominant the early time data. Papadopulos and Cooper (1967) give a criterion for estimating the amount of time that wellbore storage impacts the drawdown: $t < 25 \, r_{\rm c}^{\ 2}/T$ (modified after Weekes 1977). In this equation, $r_{\rm c}$ = radius of well casing (L), and T = transmissivity (L/T). This equation can be used to estimate when the wellbore storage is no longer dominant. In the field a unit slope on a log-log graph of the data will indicate borehole storage is dominating the drawdown data.

3.6 BASELINE AND PRE-TEST MONITORING

Before initiating the slug test and starting the constant discharge test, baseline and pre-test water-level trends must be established. A pressure transducer recording frequencies of 1 h should be used to record baseline water-level trends for 3 days or longer. Steel tapes and electric tapes used for measuring water levels must meet the calibration/ standardization requirements in EII 10.2. Barometric monitoring will also be included over the span of the testing, beginning at the time of the baseline monitoring. In addition, if time warrants, a downhole flow meter may be tested to ascertain groundwater flow direction and velocity.

3.7 POST-TEST MONITORING

After pumping is terminated, water-level data collection will continue throughout the recovery period until a dynamic equilibrium is re-established, or the recovery trend is clearly defined. In most cases full recovery is expected to occur in about 2 or 3 days. The final slug test can then be performed at the well.

3.8 CONTROL OF PURGEWATER

Purgewater will be handled in two ways, depending on the quality of the groundwater at the test well and the location of the test. Within the 200 West Area, all generated purgewater must be contained and transported to a predetermined disposal facility (WHC 1990b). Outside of the 200 West Area, if groundwater at the test well is designated as uncontaminated, the water can be

released to the ground surface. It is recommended that the water be disposed at least 100 ft away from the wellhead.

The quality of the groundwater will be documented for each well prior to testing, and thereby the proper method of disposal determined. Constant discharge tests will not be conducted at wells where the purgewater is contaminated and the hydraulic conductivity of the aquifer is relatively high, because of the logistics in containing large volumes of purgewater. Even in wells where the volume of contaminated purgewater would be small, it may not be possible to test if the purgewater cannot be disposed. A sample of the purgewater should be collected at the end of each test for information only, and analyzed at least for nitrate and total activity.

4.0 QUALITY ASSURANCE/QUALITY CONTROL

Data quality is controlled primarily by this DOW and adherence to EII 10.1 on Aquifer Testing. The data at the test wells can be reproduced if the initial test fails by re-running the test. Some of the test sites outside of the 200 West Area may require an evaluation of the impact to endangered, threatened, and sensitive species if it is decided that groundwater can be disposed to the ground.

The quality assurance documents that cover the test activities are the Quality Assurance Manual (WHC 1988d) and the Environmental Engineering, Technology, and Permitting Function Quality Assurance Program Plan (WHC 1990a). This aquifer DOW and the aquifer testing is assigned an impact level of 3Q.

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5.0 SCHEDULE

Aquifer testing is expected to begin in July 1993 and be completed in August 1993. Testing should be completed as soon as possible to allow enough time for final data reduction and analysis, and to provide input into the final modeling report that is due at the end of the fiscal year. Well assessment activities can begin any time after the applicable administrative tasks listed above are completed. Deviations from this test plan, such as using the alternate wells for testing (Table 1), will require prior regulatory approval.

6.0 REFERENCES

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- WHC, 1990b, Strategy for Handling and Disposing of Purgewater at the Hanford Site, Washington, WHC-MR-0039, Westinghouse Hanford Company, Richland, Washington.
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- WHC, 1992, Hydrogeologic Model for the 200 West Groundwater Aggregate Area, WHC-SD-EN-TI-014, Rev. 0, Westinghouse Hanford Company, Richland, Washington.

Figure 1. Location Map for the Proposed Aquifer Test Wells in the 200 West Area.

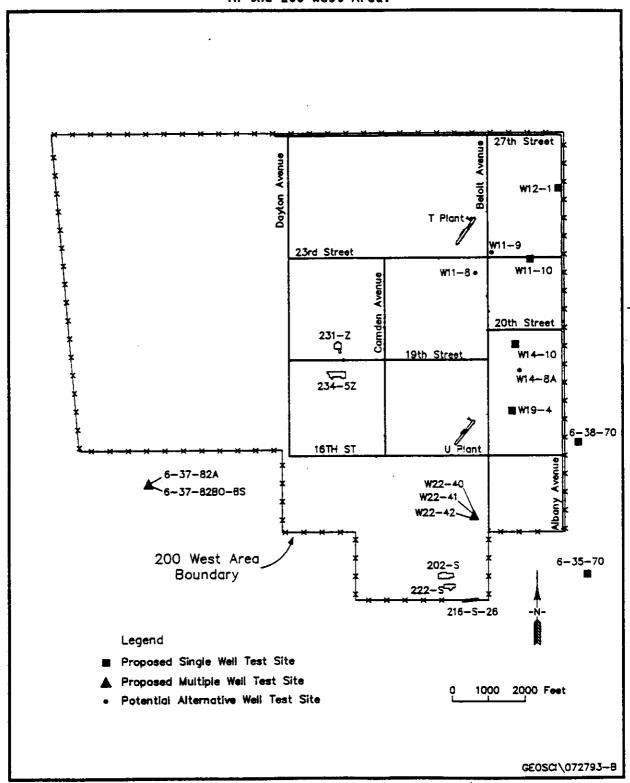
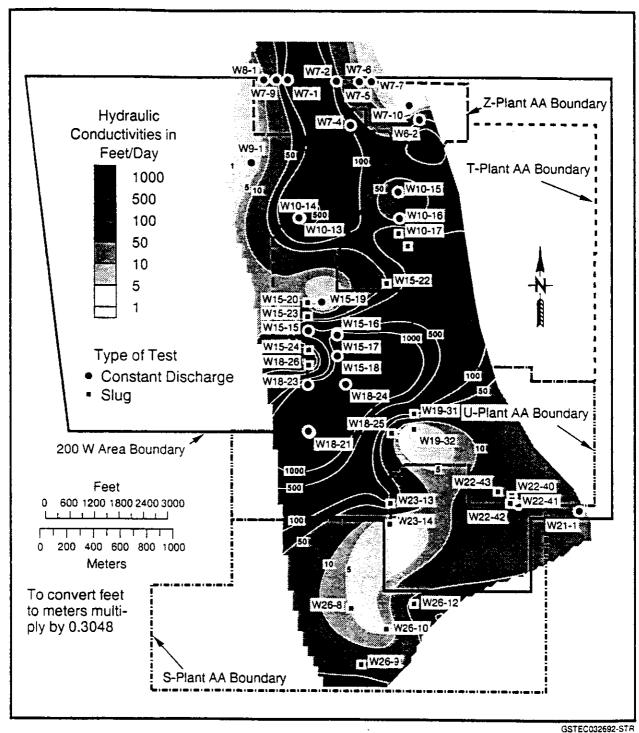


Figure 2. Hydraulic Conductivity Distribution Map of the 200 West Area (after WHC 1992).



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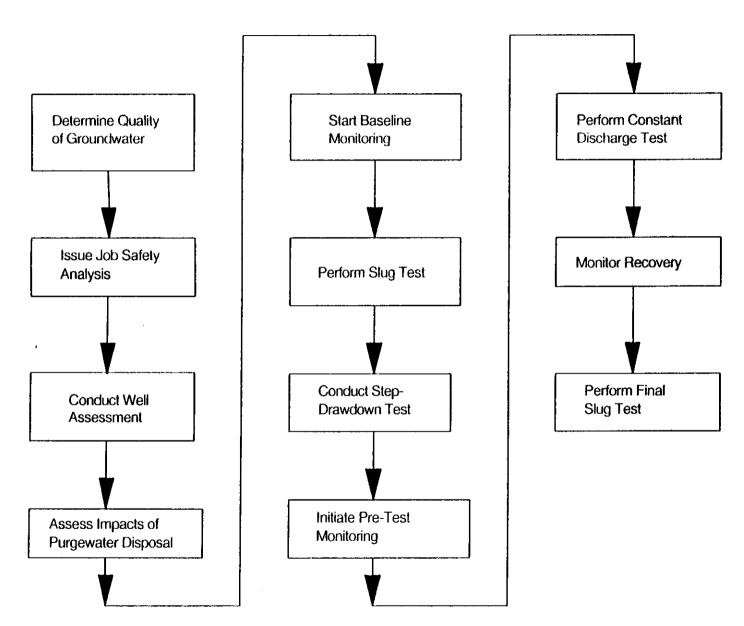
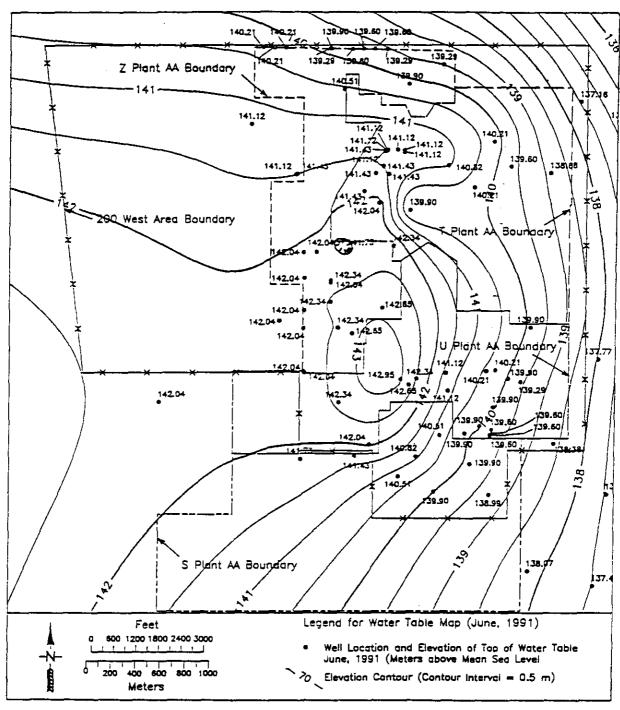


Figure 3. June 1991 Water Table Map of the 200 West Area (after WHC 1992).

Figure 4. Flow Chart Showing the Sequencing of Aquifer Test Activities.



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Well Construction Test Wells (see

Information for for locations).

the Aquifer

PERMANENT VELL F	END DRILLING DATE	COMPLETION DEPTH (ft)	DEPTH (ft)	DEPTH TO WATER (ft)	SCREEN INTERVAL (ft)	SANDPACK INTERVAL (ft)	ELEVATION BRASS CAP	CASING SIZE CIDI
299-W14-10*	JUN 81	~325	330	263, 1991	260-275P	MD	ND	8*-330 10*-242
299-W11-10*	MAR 56	305	307	270, 1991	~256-304P	NO	~726.8	8*-305
299-W12-1*	APR 56	~310	314	273, 1991	~274-309P	ND	~724.2	8*-310
299-W11-8&	NOV 53	313	315	260, 1965	260-310P	ND	ND	8×-313
299-W11-9&	FEB 54	297	297	262, 1991	6" Liner 275-297P	MD	~72 1	84-270 64270-297
299-W19-4&	FEB 60	350	550	255, 1991	255-350	MD	714	84-539
299-W14-8A+	FEB 77	ND	563	ND	ND	ND	NO	4#-196 3.5#-473
699-37-82A*	FEB 62	408?	408	ND	~155-175?	ND	636.95	1.5"
699-37-8290+1	OCT 64	184?	296?	ND	165-185	MD	ND	1.5"
699-37-828P+1	MAY 64	344?	560?	ND	540-560	NO	636.30	1.5"
699-37-8289+1	MAY 64	184	410	ND	390-410	MD	636.30	1.5*
699-37-82BR+1	MAY 64	7	330	ND	310-330	ND	636.30	1.5*
699-37-828\$+1	MAY 64	414?	2507	ND	230-250	MD	636.50	1.5*
299-W22-41*	MAY 90	245	245	231, 1991	224-245 wire wrap 5 slot	220-243 40-100	688.77	104-132 8*-244
299-W22-40+	MAY 90	244	245	231, 1992	224-244 Wire Wrap 10 slot	218-242 40	689.22	10#-143 8#-245
299-W22-42+	MAY 90	243	243	230, 1991	223-243 wire wrep 10 slot	219-243 40	688,20	10#-137 8#243
699-35-70*1	SEP 48	253	325	240, ?	235-320P 6" Liner 233-253	ND	693.72	8#-?
699-38-70*1	JUN 57	300	413	270, 1987	255-380P Plug 2300	ND	MD	84

ND=Not Documented

&=potential alternate test well

regraphic construction information not available reinterpretation of construction information is questionable: if clay cap is not found, this well will not be tested

All other wells on table are for information, due to their location near a test well.

P=Perforations

^{*=}pumping well

^{+≥}observation well

Table 2. Summary Table of Criteria for Selecting Test Wells.

Selection criteria	299- W12-1	299- W11-10	299- W14-10	299- W22-41	699- W38-70	699- W35-70	699- 37-82A
Existing Well	X	X	X	X	X	X	X
Screened at Top of Aquifer	X	X	X	X	X	X	Х
Near U-1/ U-2 Crib			X	X	Х	Х	
Confirms Conductivity				X			X
Fills Data Gap	Х	X	X		Х	X	Х
Eastern Side of 200 West	X	χ	χ		Х	Х	
Defines High Conductivity Area			Х				х
Multiple Wells				Х			X

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APPENDIX A

WELL CONSTRUCTION AND COMPLETION SUMMARIES

Most of the proposed aquifer test wells were constructed before 1981. Only one well, 299-W22-41, was completed in accordance with the Washington State Department of Ecology construction standards (Chapter 173-160 WAC, 1990). Table 1 lists information o well completions. This appendix contains as-builts and well summaries (as available) fo the proposed test wells.

Many of the wells were constructed of 8-in. carbon steel casing, which was perforated at a specific interval(s) of interest at the time of completion. One well, 299-W22-41, was completed in accordance with Washington State well construction standard Construction materials in this well included 4-in. stainless steel pipe and a continuous wire-wrap 10-slot stainless steel screen. The completion interval for this well was the top 20 ft of the unconfined aquifer.

Most of the wells were drilled using a cable tool drilling rig. Some wells have been modified through time by the addition of liners and plugs. The current status and condition of the wells will not be known until a site visit is made and in some instance a camera survey is conducted. After the well assessment, a decision will then be made o whether the well is suitable for testing. The criteria for this decision are listed in Section 3.2.

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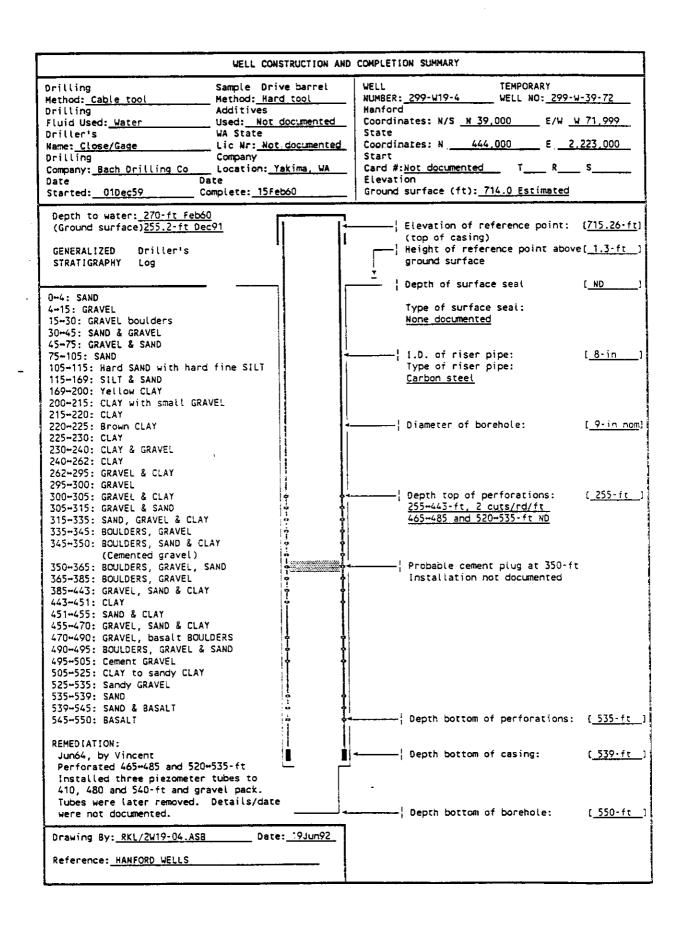
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SUMMARY OF CONSTRUCTION DATA AND FIELD OBSERVATIONS RESOURCE PROTECTION WELL - 299-W19-4

2-W19-4 WELL DESIGNATION

CERCLA UNIT 200 Aggregate Area Management Study

Not applicable RCRA FACILITY

N 39,000 W 71,999 HANFORD COORDINATES : N 444,000 E 2,223,000 LAMBERT COORDINATES :

DATE DRILLED Feb60 550-ft DEPTH DRILLED (GS) :

MEASURED DEPTH (GS) : Not documented DEPTH TO WATER (GS) : 270-ft Feb60; 255.2-ft, Dec91

8-in, +1.3+539-ft CASING DIAMETER ELEV TOP CASING 715.26-ft

714.0-ft, Estimated ELEV GROUND SURFACE : 255-443 and 465-485-ft PERFORATED INTERVAL : Not applicable SCREENED INTERVAL

FIELD INSPECTION, 09May91, COMMENTS

8-in carbon steel casing.

No pad, No posts, capped and locked. Identification stamped on brass cap.

Not in radiation zone.

OTHER: Well formerly contained five 1½-in piezometers, 299W-19-40,P,Q,R and S. All were removed, date not documented. Well apparently now has a cement plug at ~350-ft.

AVAILABLE LOGS Driller Not applicable TV SCAN COMMENTS Not applicable DATE EVALUATED EVAL RECOMMENDATION : Not applicable

Separations area water level measurement, 15Jan64~01Dec91 LISTED USE

Not on water sample schedule

PUMP TYPE None documented

MAINTENANCE

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SUMMARY OF CONSTRUCTION DATA AND FIELD OBSERVATIONS RESOURCE PROTECTION WELL - 299-W14-8A

299-W14-8A WELL DESIGNATION

CERCLA UNIT 200 Aggregate Area Management Study

RCRA FACILITY

Not applicable N 40,105 W 71,788 N 449,222 E 2,223,434 HANFORD COORDINATES : LAMBERT COORDINATES :

DATE DRILLED Feb77 563-ft DEPTH DRILLED (GS) :

MEASURED DEPTH (GS) : Not documented DEPTH TO WATER (GS) : Not documented

6-in carbon steel, T+1.0-196-ft; 4-in carbon steel, T+1-196-ft; 3½-in carbon steel, T+1-473-ft CASING DIAMETER

723.48-ft ELEV TOP CASING

ELEV GROUND SURFACE : 721.3-ft, Estimated PERFORATED INTERVAL : 0+20-ft

SCREENED INTERVAL Not applicable

FIELD INSPECTION, 22Apr91, COMMENTS

6, 4 and 3%-in carbon steel casings. No pad, No posts, not capped, not locked.

1½-in PVC pipe stuck in hole. No permanent identification.

OTHER: Completed as OH-13A corehole.

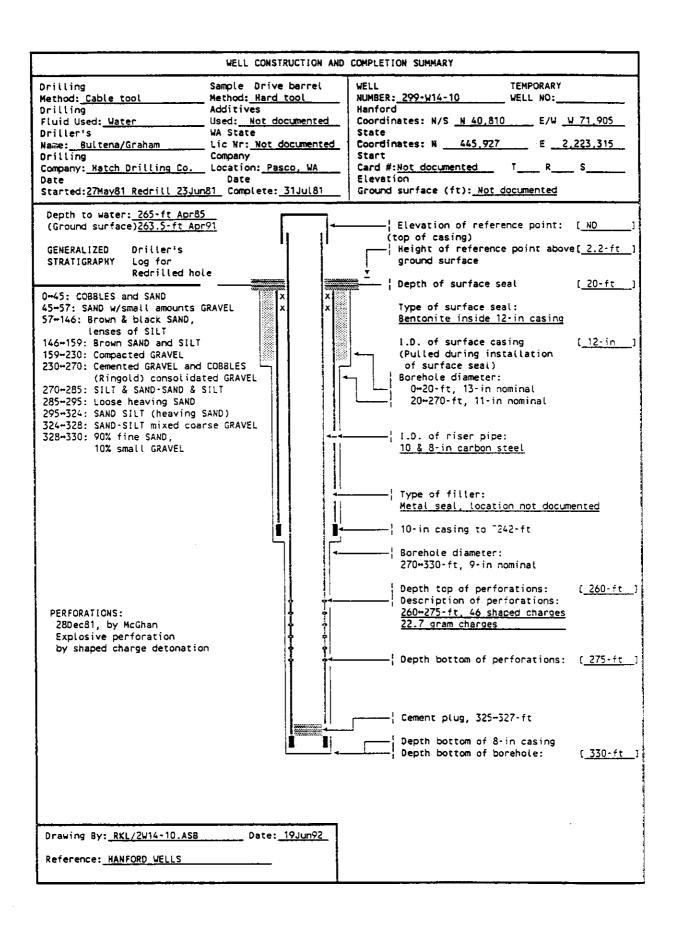
Driller/Geologist AVAILABLE LOGS TV SCAN COMMENTS Not applicable DATE EVALUATED Not applicable Not applicable EVAL RECOMMENDATION : JISTED USE No water level data

Not on water sample schedule

FUMP TYPE None documented

MAINTENANCE

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SUMMARY OF CONSTRUCTION DATA AND FIELD OBSERVATIONS RESOURCE PROTECTION WELL - 299-W14-10

WELL DESIGNATION : 299-W14-10

CERCLA UNIT : 200 Aggregate Area Management Study

RCRA FACILITY : Not applicable HANFORD COORDINATES: N 40,810 W 71,905 LAMBERT COORDINATES: N 445,927 E 2,223,315

DATE DRILLED : Jul81
DEPTH DRILLED (GS) : 330-ft
MEASURED DEPTH (GS) : 315.5-ft, Apr91
DEPTH TO WATER (GS) : 265-ft, Aug85;
263.5-ft, Apr91

CASING DIAMETER : 10-in carbon steel, 0-242-ft; 8-in carbon steel, +2.9-330-ft

ELEV TOP CASING : Not documented ELEV GROUND SURFACE : Not documented PERFORATED INTERVAL : 260+275-ft SCREENED INTERVAL : Not applicable

COMMENTS: FIELD INSPECTION, 09May91, 8-in carbon steel casing.

2-ft cement pad, No posts, capped and locked.
Well identification stamped on brass cap in pad.

Not in radiation zone.

OTHER: Perforation done by explosive shaped charges.

AVAILABLE LOGS : Driller
TV SCAN COMMENTS : Not applicable
DATE EVALUATED : Not applicable
EVAL RECOMMENDATION : Not applicable

LISTED USE : Water levels measured, O8Mar82+12Apr90

PML Semis-nual, WHC Quarterly water sample schedule

PUMP TYPE : Electric submersible

MAINTENANCE :

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SUMMARY OF CONSTRUCTION DATA AND FIELD OBSERVATIONS RESOURCE PROTECTION WELL - 299-W11-10

WELL DESIGNATION : 299-W11-10

CERCLA UNIT : 200 Aggregate Area Management Study

RCRA FACILITY: Not applicable
HANFORD COORDINATES: N 43,150 W 71,500
LAMBERT COORDINATES: N 448,268 E 2,223,714
DATE DRILLED: Apr56

DATE DRILLED : Apr56
DEPTH DRILLED (GS): 307-ft
MEASURED DEPTH (GS): Not documented
DEPTH TO WATER (GS): 279-ft, Apr56;
270.6-ft, Dec91

CASING DIAMETER : 8-in carbon steel, +2.1="305.0-ft;

ELEV TOP CASING : 728.89-ft

ELEV GROUND SURFACE: 726.8-ft, Estimated
PERFORATED INTERVAL: 8-in casing, 256-304-ft

SCREENED INTERVAL : Not applicable

COMMENTS: FIELD INSPECTION, 21May91, 8-in carbon steel casing.

No pad, No posts, capped and locked.

No permanent identification.

Not in radiation zone.

OTHER:

AVAILABLE LOGS : Oriller

TV SCAN COMMENTS : Not applicable DATE EVALUATED : Not applicable EVAL RECOMMENDATION : Not applicable

LISTED USE : Separations area Semiannual water level measurement, 18Apr56-01Dec91

Not on water sample schedule

PUMP TYPE : None documented

MAINTENANCE :

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	WELL CONSTRUCTION AND	COMPLETION SUMMARY	
Date Da	Sample Method: Hard tool nom Additives Used: Not documented WA State Lic Nr: Not documented Company Location: Not documented te mplete: 09May56	WELL TEMPORARY NUMBER: 299-W12-1 WELL NO: 361 Hanford Coordinates: N/S N 45.083 E/W W State Coordinates: N 450.203 E Z. Start Card #:Not documented T R Elevation Ground surface (ft): 724.2 Estimated	70,733 224,476
Depth to water: 288-ft May56 (Ground surface)273.1-ft Dec GENERALIZED Driller's STRATIGRAPHY Log 0+10: TOPSOIL-GRAVEL 10+21: SAND-COBBLES 21+72: Not documented 72+110: Pea GRAVEL-SAND-SILT 110+127: Not documented		Elevation of reference point:	[<u>2,26-ft</u>]
127-136: GRAVEL, SAND, SILT 144,148: SAND 148-175: Not documented 175-178: SAND-GRAVEL 179,187: BCULDERS, SAND and G 187-200: Not documented 200-230: Cemented GRAVEL 230-297: Not documented 297-314 SAND, SILT and GRAVE	4.	(Assumed pullied)	[8-in]
		Depth top of perforations: Description of perforations: Not documented	(<u>274-ft</u>)
		Depth bottom of perforations: Depth bottom of casing: Depth bottom of borehole:	(309-ft) (310-ft)
Drawing By: RKL/2W12-01.ASB Reference: HANFORD WELLS	Date: 19Jun92		

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SUMMARY OF CONSTRUCTION DATA AND FIELD OBSERVATIONS RESOURCE PROTECTION WELL - 299-W12-1

299-W12-1 WELL DESIGNATION

CERCLA UNIT 200 Aggregate Area Management Study

RCRA FACILITY

Not applicable N 45,083 W 70,733 N 450,203 E 2,224,476 HANFORD COORDINATES : LAMBERT COORDINATES :

DATE DRILLED May56 DEPTH DRILLED (GS) : 314-ft

MEASURED DEPTH (GS) : Not documented 288-ft, May56 273.1-ft, 01Dec91 DEPTH TO WATER (GS) :

CASING DIAMETER 8-in carbon steel, +2.3+~310-ft;

ELEV TOP CASING 726.46-ft

ELEV GROUND SURFACE : 724.2-ft Estimated PERFORATED INTERVAL : 8-in casing, 274-309-ft

SCREENED INTERVAL : Not applicable

FIELD INSPECTION, 22Apr91, COMMENTS 8-in carbon steel casing.

No pad, No posts, capped, not locked.

No permanent identification.

Not in radiation zone.

OTHER:

Driller AVAILABLE LOGS TV SCAN COMMENTS : DATE EVALUATED - : Not applicable Not applicable EVAL RECOMMENDATION : Not applicable

Separations area water level measurement, 21May56+01Dec91 LISTED USE

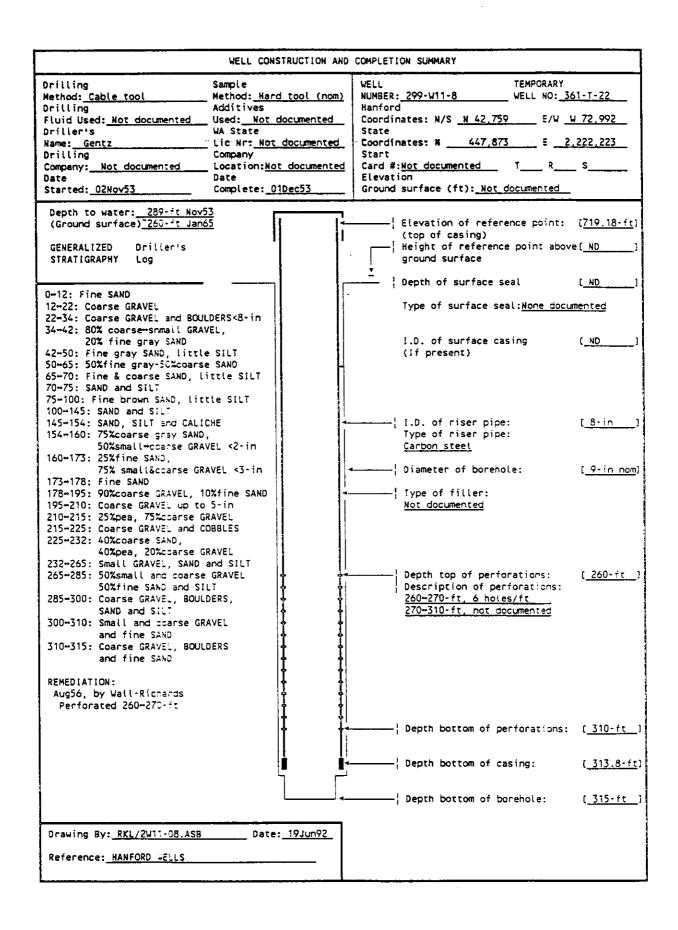
PNL Annual water sample schedule

PUMP TYPE None documented

MAINTENANCE

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SUMMARY OF CONSTRUCTION DATA AND FIELD OBSERVATIONS RESOURCE PROTECTION WELL - 299-W11-8

WELL DESIGNATION : 299-W11-8

CERCLA UNIT : 200 Aggregate Area Management Study

RCRA FACILITY : HANFORD COORDINATES : N 42,759 W 72,992 LAMBERT COORDINATES : N 447,873 E 2,222,223

DATE DRILLED : Dec53
DEPTH DRILLED (GS): 315-ft
MEASURED DEPTH (GS): Not do

MEASURED DEPTH (GS): Not documented DEPTH TO WATER (GS): 289-ft, Sep56; -260-ft, Jan65

CASING DIAMETER : 8-in carbon steel, +ND+"314-ft;

ELEV TOP CASING : 719.18-ft
ELEV GROUND SURFACE : Not documented
PERFORATED INTERVAL : 8-in casing, 26

PERFORATED INTERVAL : 8-in casing, 260+310-ft SCREENED INTERVAL : Not applicable

COMMENTS : FIELD INSPECTION, OTHER:

AVAILABLE LOGS : Driller
TV SCAN COMMENTS : Not applicable
DATE EVALUATED : Not applicable
EVAL RECOMMENDATION : Not applicable

LISTED USE : Water levels measured, 21Jan54-19Jan65;

Not on water sample schedule

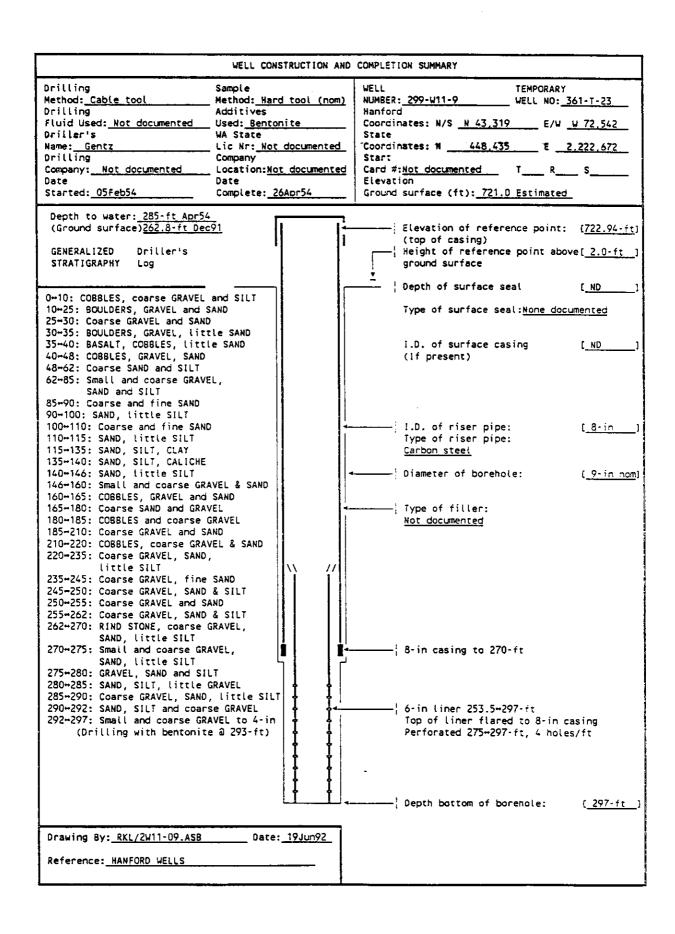
PUMP TYPE : None documented

MAINTENANCE

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SUMMARY OF CONSTRUCTION DATA AND FIELD OBSERVATIONS RESOURCE PROTECTION WELL - 299-W11-9

299-W11-9 WELL DESIGNATION

CERCLA UNIT 200 Aggregate Area Management Study

RCRA FACILITY HANFORD COORDINATES : N 43,319 W 72,542

N 448,435 E 2,222,672 LAMBERT COORDINATES :

DATE DRILLED Apr54 DEPTH DRILLED (GS) : 297-ft

MEASURED DEPTH (GS) : Not documented 285-ft, Apr54; DEPTH TO WATER (GS) : 262.8-ft, 01Dec91

8-in carbon steel, +2.3-270-ft; 6-in carbon steel lines, 253.5+297-ft CASING DIAMETER

ELEV TOP CASING 722.94-ft ELEV GROUND SURFACE : Not documented

PERFORATED INTERVAL : 6-in liner, 275+297-ft

SCREENED INTERVAL : Not applicable

COMMENTS -FIELD INSPECTION, 21May91, 8-in carbon steel casing.

No pad, No posts, capped and locked.

No permanent identification. Not in radiation zone.

OTHER:

AVAILABLE LOGS Driller

TV SCAN COMMENTS : Not applicable DATE EVALUATED Not applicable EVAL RECOMMENDATION : Not applicable

LISTED USE Separations area Semiannual water level measurement, 17Jun54+01Dec91; :

PNL Annual water sample schedule

PUMP TYPE None documented

MAINTENANCE

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WELL CONSTRUCTS	ON AND	COMPLETI	ON SUMMARY	
Drilling Sample Drive barn Method: Cable tool Method: Hard tool Drilling Additives Fluid Used: Water Used: None Driller's WA State Name: L. Watkins Lic Nr: Not docume Drilling Company Company: Kaiser Engineers Han Location: Richland Date Started: D1Mar90 Complete: 15May90	ented	Hanford Coordin State N Coordin Start Card #: Elevati	Nates: N/S <u>N 36,242.3</u> E/W <u>W</u> NAD83 Nates: N <u>134,509.99m</u> E <u>56</u>	7,634.57m
Depth to water: 228.1-ft Apr90 (Ground surface)231.8-ft Mar92 GENERALIZED Geologist's STRATIGRAPHY Log 5: SAND 10: Slightly gravelly, slightly muddy SAND 15: Slightly muddy SAND 20: Muddy SAND 20: Muddy SAND 25,30: Slightly gravelly SAND 35,40: SAND 43: Muddy SAND 43: Muddy SAND 60: Slightly gravelly SAND 60: Slightly gravelly SAND 64: Slightly muddy SAND 65: Gravelly SAND 70,75,80,85: SAND		•	Elevation of reference point: (top of casing) Height of reference point above ground surface Depth of surface seal Type of surface seal: Cement grout	
87: Slightly muddy SAND 90,95: SAND (Sl muddy 92-93, 94-94.5) 100: Muddy SAND 105: SAND 110,115: Slightly muddy SAND 120+155: SAND 160,165,170: Slightly muddy SAND 175: Muddy SAND 180: Slightly muddy SAND 180: Slightly muddy SAND 190: Slightly gravelly, slightly muddy SAND 195: Slightly muddy gravelly SAND 200: Slightly muddy SAND 205: Slightly muddy SAND 205: Slightly muddy SAND			10 3/4-in nominal hole to 143. I.O. of riser pipe: Type of riser pipe: Type 304 stainless steel Diameter of borehole: 143.5*245.0-ft Type of filler, 20.5*211.1-ft: Bentonite crumbles 8*20 mesh	[<u>4-in</u>]
muddy SAND 210,215,220: Muddy SAND 225: Slightly muddy SAND 230: Muddy SAND 235,240,245: Muddy sandy GRAVEL			Bentonite pellets, 1/2-3/8-in 211.1+218.1-ft Depth top of sand pack: 40 mesh silica sand to 243.2-ft Depth top of screen: 4-in, #10-slot, Johnson type	
			stainless steel, wire wrapper with bottom cap and channel; Depth bottom of screen: Depth bottom of borehole:	
Drawing By: RKL/2W22-40.ASB Date: 19Jun Reference: WHC-MR-0208, October 1990 KEH Survey Data Report, 18Jun90	192			

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SUMMARY OF CONSTRUCTION DATA AND FIELD OBSERVATIONS RESOURCE PROTECTION WELL - 299-W22-40

299-W22-40 WELL DESIGNATION :

CERCLA UNIT 200 Aggregate Area Management Study

RCRA FACILITY 216-0-12

N 36,242.3 W 73,041.7 HANFORD COORDINATES :

LAMBERT COORDINATES : NAD83 N 134,509.99m E 567,634.57m

DATE DRILLED May90 DEPTH DRILLED (GS) : MEASURED DEPTH (GS) : 245.0-ft 244.5-ft

DEPTH TO WATER (GS) :

228.1-ft, Apr90; 231.8-ft, Mar92 4-in, stainless steel, +ND+224.1-ft; CASING DIAMETER

6-in, stainless steel, +3.0+T0.5-ft (not documented)

ELEV TOP CASING 692.33-ft

ELEV GROUND SURFACE : 689.22 (Brass cap) PERFORATED INTERVAL : Not applicable

SCREENED INTERVAL : 224.1-244.5-ft, #10-slot, stainless steel

COMMENTS FIELD INSPECTION.

OTHER: AVAILABLE LOGS Driller

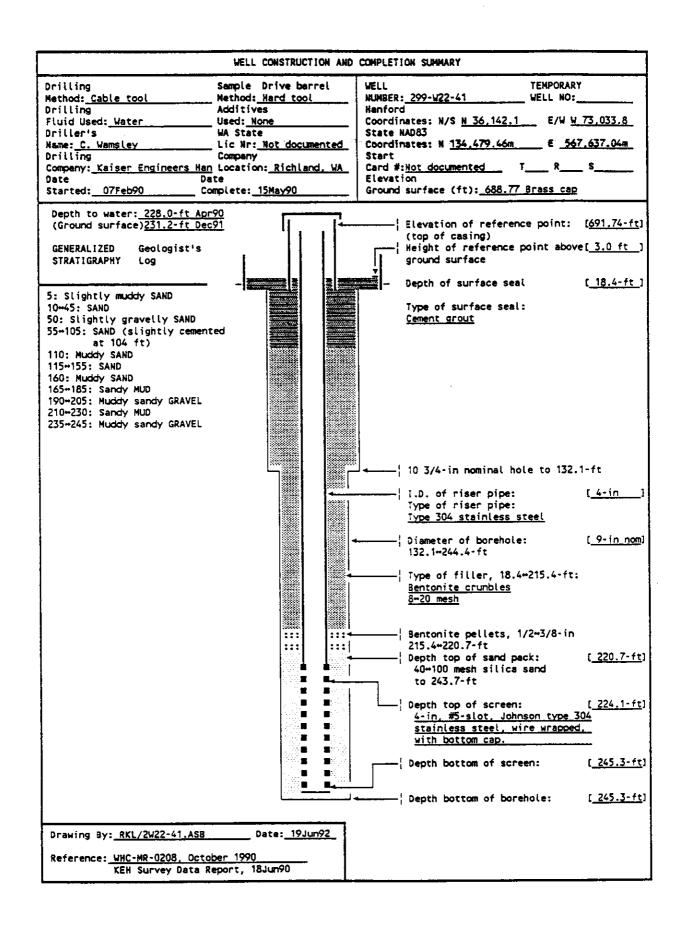
TV SCAN COMMENTS Not applicable : DATE EVALUATED Not applicable EVAL RECOMMENDATION : Not applicable

U-12 crib Quarterly water level measurement, 20Nov90-11Mar92; LISTED USE :

Not on water sample schedule

PUMP TYPE Hydrostar

MAINTENANCE



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- Page 1

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SUMMARY OF CONSTRUCTION DATA AND FIELD OBSERVATIONS RESOURCE PROTECTION WELL - 299-W22-41

299-W22-41 WELL DESIGNATION

200 Aggregate Area Management Study CERCLA UNIT

216-0-12 RCRA FACILITY

N 36,142.1 W 73,033.8 HANFORD COORDINATES :

NAD83 N 134,479.46m E 567,637.04m LAMBERT COORDINATES :

DATE DRILLED May90 245.3-ft DEPTH DRILLED (GS) : MEASURED DEPTH (GS) : 245.3-ft

DEPTH TO WATER (GS) :

CASING DIAMETER

228.0-ft, Apr90; 231.3-ft, Mar91 4-in, stainless steel, +ND=224.1-ft; 6-in, stainless steel, +3.0=0.5-ft (not documented) 691.74-ft

ELEV TOP CASING

688.77 (Brass cap) ELEV GROUND SURFACE : PERFORATED INTERVAL : Not applicable

224.1+245.3-ft, #5-slot, stainless steel SCREENED INTERVAL :

FIELD INSPECTION; COMMENTS

OTHER: Driller AVAILABLE LOGS

Not applicable TV SCAN COMMENTS DATE EVALUATED Not applicable EVAL RECOMMENDATION : Not applicable

U-12 Crib Quarterly water level measurement, 20Nov90-11Mar91; LISTED USE

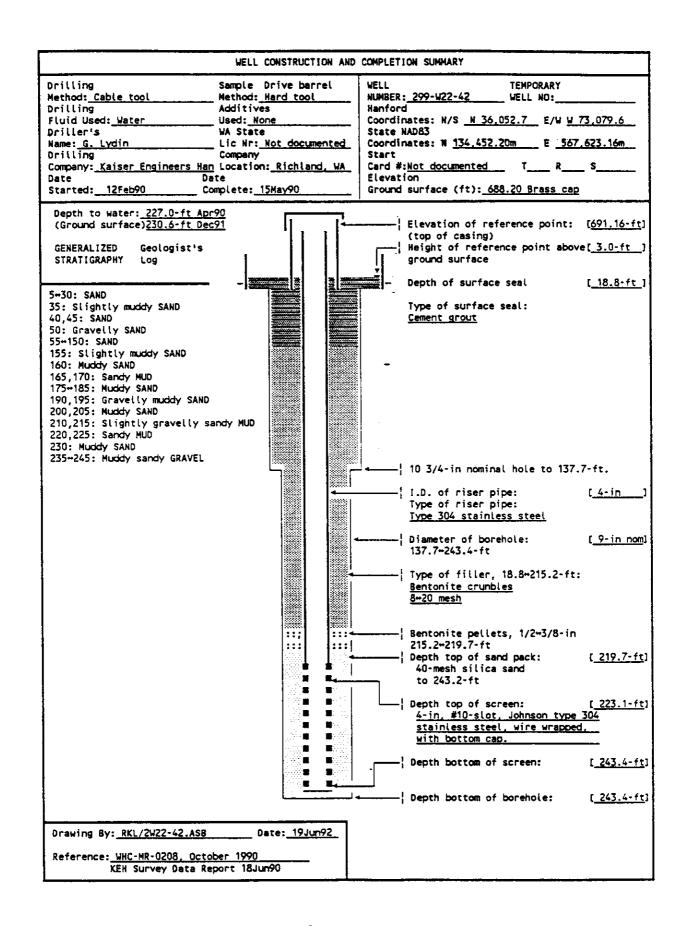
Not on water sample schedule

Hydrostar PUMP TYPE

MAINTENANCE

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SUMMARY OF CONSTRUCTION DATA AND FIELD OBSERVATIONS RESOURCE PROTECTION WELL - 299-W22-42

WELL DESIGNATION : 299-W22-42

CERCLA UNIT 200 Aggregate Area Hanagement Study :

RCRA FACILITY : 216-U-12

HANFORD COORDINATES : N 36,052.7 W 73,079.6

MAD83 N 134,452.20m E 567,623.16m LAMBERT COORDINATES :

DATE DRILLED May90 DEPTH DRILLED (GS): MEASURED DEPTH (GS): 243.4-ft 243.4-ft DEPTH TO WATER (GS) : 227.0-ft, Apr90;

230.6-ft, Dec91
4-in, stainless steel, "+1.0+223.1-ft;
6-in stainless steel, +3.0+0.5-ft (not documented) CASING DIAMETER

ELEV TOP CASING 691.16-ft

ELEV GROUND SURFACE : 688.20 (Brass cap) PERFORATED INTERVAL : Not applicable

SCREENED INTERVAL : 223.1-243.4-ft, 10-slot, stainless steel

COMMENTS FIELD INSPECTION:

OTHER: AVAILABLE LOGS Driller

TV SCAN COMMENTS Not applicable DATE EVALUATED Not applicable EVAL RECOMMENDATION : Not applicable

LISTED USE Water levels measured, 20Nov90+10Dec91;

Not on water sample schedule

PUMP TYPE Hydrostar

MAINTENANCE

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Drilling Method: <u>Cable tool</u>	Sample Method: <u>Hard_tool (nom)</u>	WELL TEMPORARY NUMBER: 699-38-70 WELL NO:	
Drilling	Additives	Hanford	
Fluid Used: Water	Used: Bentonite	Coordinates: N/S N 38,142 E/W W	70.226
Driller's	WA State	State	10,200
Name: Swain	Lic Nr: Not documented		225,001
Drilling	Company	Start	
Company: Not documented	Location: <u>NO</u>	Card #: Not documented T R	s
	Date	Elevation	
Started: 17May57	Complete: 14Jun57	Ground surface (ft): Not documented	
Depth to water: 270-ft Jun	87	I flowation of majorana points	*710 47_£
			(/1U.0/-TT
GENERALIZED Driller's	ч и	Height of reference point above	יו אט
STRATIGRAPHY Log	1 1	ground surface	I NO
STRAITERAPHI EUG	1 1	A ALOUNG 2011ACA	
	- — _г	Depth of surface seal	(ND
0-5: SAND		Time of minface seals	
5-15: Small GRAVEL		Type of surface seal:	
15-25: Sandy SILT-GRAVEL		None documented	
25-30: SILT, coarse SAND	11 11	I.D. of surface casing	r un
30+45: Sandy SILT 45+55: SAND-SILT-GRAVEL		(If present)	(<u>ND</u>
45→55: SAND-SILI-GRAVEL 55→70: SAND-SILT	H H	(1) presency	
70→72: SAND-STEI 70→72: SAND-SMAIL GRAVEL	11 (2	DRILLER'S NOTE: Casing may be	
72-80: SAND-coarse and clea	. H 16	parted at joint 50 ft from top	
80÷120: SAND-SILT	" !! !!	her see as farms as in many rate	
120-130: SAND-SILT (harder	nacked)		
130-190; SAND-SILT			[8-in
190-200: SAND-SILT-small GR	AVFI (water)	Type of riser pipe:	1 -4-11
200-205: SAND-SILT-small GR		Carbon steel	
205-220: SAND-SILT	^ ``		
220-230 SAND-SILT, soft, mo	ITE CLAY	Diameter of borehole:	[_9-in_no
than SAND		•	•
230-245: Small GRAVEL-CLAY	<u> </u>		
245-250: CLAY		Not documented	
250+260: CLAY-GRAVEL	- 11 (1		
260+265: SAND-SILT-GRAVEL		Elevation/depth top of seal	
265-310: SAND-GRAVEL, most		Type of seal: Not documented	
310+315: SAND-GRAVEL, a lit	tle SILT		
315-320: SAND-SILT-GRAVEL-C		Depth top of perforations:	[<u>255-ft</u>
320+335: SAND, SILT and GRA	·VEL 🛉 👣	255+320-ft, 3 cuts/ft	
335-345: SAND-GRAVEL	† †	320+380-ft, 2 cuts/2 ft	
345-350: Clean coarse SAND	† †		
350-360: Fine clean SAND	IÎ II		
360+365: SAND	Î Î		
365+369: SAND-small GRAVEL	ÎI		
369+375; SAND-hardpacked		 	
375→380: SAND-softer, very 380→390: SAND-SILT, very fi	Tine I	cement plug, 300-310-10	
390-395: SAND-SILI, VERY 11	^{1 [[] [] [] [] [] [] [] [] []}		
395-400: SAND	I L	<pre>↓ Depth bottom of perforations:</pre>	r 780-ft
400+413: Fine SAND and SIL	, T	peptil boctom or personational	[<u>]00 ; ;</u>
(caving)	Fill ?		
REMEDIATIONS:		Depth bottom of casing:	[_388-ft
Jun 64, Crowe		paper parroll or annual	<u> </u>
Installed plastic piezon	neter tubes		
Jul75, M. Bultena, clear	neter tupes ned well		
Jul77, Bigham, set cemer		Depth bottom of borehole:	[_413-ft
		,	* <u></u>
Drawing By: RKL/6N38W70.ASE	Date: 29Jun92		
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ME 639-37-82A 175.00 Feet PPH 440.00 Feet NELL CONSTRUCTION * LITHOLOGY-D CLS VELL CONSTR	639-37-82A COMPLETICN DATE 10/10/ 636.75 Feet 175.00 Feet 37018 P E-W	WHC-SD-EN-AP-130, Rev. 1 699-37-82A 636.75 Feet 175.00 Feet 270.18 P E-W 370.18 P CONSTRUCTION *LITHOLOGY-DICLS -81988 P PAGE 1 of		1				
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